

**VALUE ENGINEERING DISCUSSIONS
I-471 PAVEMENT REHABILITATION
CAMPBELL COUNTY
ITEM NUMBER 6-2021.00
NOVEMBER 1, 2011**

INTRODUCTION

This report summarizes the results of a Value Engineering Study conducted by the project development team on August 25, 2011. The subject of the study was the pavement rehabilitation of I-471 in Campbell County.

The proposed letting for this project is February 24, 2012 with a final plan submittal date of January 2, 2012.

PROJECT DESCRIPTION

This project will replace the existing pavement on I-471 from MP 0.0 (I-471/US 27 intersection) to the Ohio River at MP 5.5. This particular section of interstate intersects I-275, US 27, Grand Avenue (Ky. 1892), Memorial Parkway (Ky. 1120), and Ky. 8. The typical section varies directionally from two to four lanes and due to deteriorating pavement conditions the cabinet has decided to move forward with a pavement rehabilitation project to enhance pavement ride. In addition, this project will replace substandard guardrail, upgrade substandard guardrail end treatments, enhance bridge deck quality, replace lighting, and widen the I-471 southbound ramp to I-275 westbound from one lane to two for additional capacity and improved operation. All work on this project will be completed within the existing Right of Way with minimal impact to utilities.

METHODOLOGY

The project design team identified four major topics that made up a majority of the project cost and were worthy of value engineering considerations. This report summarizes the results of the Value Engineering Study conducted by the project design team on August 25, 2011.

The evaluation criteria identified as a basis for the comparison of alternatives on this project included the following:

- Construction Cost
- Service Life
- Traffic Control
- Future Maintenance Cost
- Future Traffic Flow

PROJECT TEAM MEMBERS

The following project development team members were present and participated in the Value Engineering discussions associated with this project.

| | |
|------------------|----------------------------------|
| James Napier | WMB, Inc. |
| Robert Franxman | KYTC D-6 Construction |
| Dan Hite | KYTC CO Design |
| Kevin Martin | KYTC CO Design |
| David Tipton | KYTC CO Maintenance |
| Robert Pennell | KYTC CO Maintenance |
| David Hamilton | KYTC CO Design |
| Danny Molen | KYTC CO Geotech. |
| Daryl Greer | KYTC CO Geotech. |
| Andre Johannes | KYTC CO Design |
| Paul Looney | KYTC CO Design |
| Dan Byers | WMB, Inc. |
| Wallace Bennett | WMB, Inc. |
| Jack Stewart | WMB, Inc. |
| Michael Loyselle | FHWA |
| Rob Harris | KYTC CO Construction |
| Gary Raymer | KYTC CO Quality Assurance |
| Bob Hill | KYTC D-6 Work Zone Safety Coord. |
| Larry Trenkamp | KYTC D-6 Construction |
| George Hoffman | KYTC D-6 Design |
| Greg Kreutzjans | KYTC D-6 Construction |
| Brandon Seiter | KYTC D-6 Bridge Maintenance |
| Rob Hans | KYTC D-6 Chief District Engineer |
| Bob Yeager | KYTC D-6 Design |
| Todd Von Behren | WMB, Inc. |
| Rick Davis | KYTC D-6 Engineering Support |

In order to have an unbiased view of the project approach, the project team solicited the assistance of 2 outside members with construction and design experience to participate in the Value Engineering Study as related to this project. These two members had no prior knowledge of the project scope or the intended project outcome. The two members were Gary Raymer of the KYTC quality assurance branch and Kevin Martin of KYTC Highway Design Branch. Gary provided valuable insight and background into construction approach during VE discussions while Kevin provided a new perspective on Ramp A widening discussions (I-471SB to I-275 WB). Both Kevin and Gary are knowledgeable in value engineering techniques and procedures.

MAJOR DISCUSSION TOPICS

The following topics account for the majority of the project cost and therefore were discussed at length during the Value Engineering deliberations within the project team meeting.

Pavement Design

Two pavement designs were considered for Value Engineering Discussions on this project. These two alternates are as follows:

Alternate 1: 15.75 inches of Flexible (Bituminous) Pavement

11.5 inches of CL4 ASPH BASE 1.0D PG 64-22
3.0 inches of CL4 ASPH BASE 1.0D PG 76-22
1.25 inches of CL4 ASPH SURF 0.38A PG 76-22

Alternate 2: 13.00 inches of Rigid (PCC) Pavement

13" JPC Pavement

Both alternates will be stabilized with 4 inches of crushed stone base on 12 inches of cement stabilized roadbed. In addition, both alternates will include an edge drain system with 4" perforated pipe on the inside and outside shoulders. Experience with edge drain systems indicate an outlet spacing of 250 to 500 feet is effective in removing the water from the pavement, especially when one outlet becomes clogged. The two alternates were discussed by the project team with various evaluation criteria in mind. The construction or initial cost favors the concrete option with a 10.6 percent savings while the Life Cycle Cost alternate favors the concrete option with a 12.1 percent savings. The initial cost and Life Cycle cost (assuming a 4% discount factor) for each alternate are as follows:

| | <u>Initial Cost</u> | <u>Life Cycle Cost</u> |
|-------------------|---------------------|------------------------|
| Rigid Pavement | \$17.40 Million | \$18.90 Million |
| Flexible Pavement | \$19.24 Million | \$21.18 Million |

See Figure A (attached) for a detailed cost analysis.

These two pavement designs are structurally equivalent, and were designed to carry the traffic loading for forty years. Past experience in the area indicates that the flexible alternate will require three rehabilitation cycles over the forty year life due to rutting and shoving associated with heavy truck traffic. Therefore, a rehabilitation cycle consisting of milling and resurfacing at ten year

intervals to enhance ride were selected for the flexible alternate. The rigid pavement will require rehabilitation cycles consisting of cleaning and resealing the joints along with diamond grinding at year 15 and year 30 in the pavements fatigue life. The attached life-cycle cost analysis was developed for discount rates of 0%, 2%, 4%, 6%, 8% and 10%. The discount rate reflects the difference between the interest rate and the inflation rate.

Although user costs were not of great consideration when discussing pavement type, figure A depicts general costs associated with future rehabilitation cycles. As the figure illustrates, it is reasonable to expect the Life Cycle Cost of the flexible pavement to be higher than the rigid alternate due to the additional rehabilitation cycle.

The current traffic using this section of urban interstate is in excess of 97,000 ADT. Currently there are no plans for widening this facility in the immediate future. Therefore, an objective in developing the pavement design was to develop a pavement rehabilitation strategy which would be durable and long lasting with a minimum number of future restoration, resurfacing or rehabilitation cycles. This is why a forty year design and analysis period was selected for this project.

Other Principal and Secondary factors as outlined in Appendix B of the AASHTO Pavement Design Guide that were considered important in selecting the pavement type are outlined below.

Principal Factor--Soil Characteristics:

It was discussed that PCC pavement might possibly be better for bridging weak soils or highly moisture sensitive soils that might be encountered. However, since mainline I-471 is designed for cement stabilized roadbed, this is not considered to be a great advantage.

Principal Factor--Construction Considerations:

Construction considerations were generally considered to favor asphalt pavements. This was due largely to the belief that asphalt pavements could be placed more quickly than PCC pavement and was not subject to greater curing considerations as was PCC pavements. However, the I 275 warranty project demonstrated that with an A + B bidding scenario, the PCC contractor bid less time to complete the project than the asphalt bidder and actually completed the project earning an incentive. Thus, construction time might not be a factor.

Secondary Factor--Availability of Local Materials or Contractor Capabilities:

There are several contractors (both asphalt and PCC) within the geographical location of this project that have the knowledge and capability of constructing this project. An abundant supply of materials exists to support either a PCC pavement contractor or an asphalt contractor. Therefore it was concluded that no advantage exists relative to an asphalt pavement versus a PCC pavement for this particular project.

Secondary Factor--Stimulation of Competition:

Stimulation of competition is desirable, especially where the potential for lack of competition exists. Kentucky has used alternate bidding on previous projects in an effort to demonstrate that alternate bidding can stimulate competition. These projects were considered very successful in drawing interest from additional contractors that may not have submitted a bid if only one pavement alternate were presented.

A Concrete overlay option was briefly discussed, but with the difference in construction cost of only 8 percent (See Figure B) and the fact that the team felt the problem with the existing pavement is found in the poor subgrade, it was determined that the best option was to utilize full depth replacement throughout the project thereby insuring a stabilized subgrade.

At the conclusion of the pavement discussion, the Value Engineering slightly favored the concrete alternate. This decision was primarily based on the fact that the surrounding pavement in the area was concrete, but felt that an asphalt alternate may have possibilities. Figure A illustrates cost analysis of the two alternates.

A subsequent meeting held by the project team in Covington on October 25, 2011 opened the discussion of pavement type selection again. While some factors pointed toward the selection of asphalt pavement, others lend themselves to the choice of a concrete pavement. While both the initial cost and life cycle cost favors the concrete alternate, the life cycle cost is within the comparable definition of 20 percent as outlined in Chapter 4 of the Alternate Pavement Bidding Document under Appendix E of the Kentucky Department of Transportation's Pavement Type Selection Policy.

In addition, there has been no overwhelming factor identified that clearly indicates a definite choice of one pavement type over the other. Geographical conditions are such that either pavement could be constructed. Also, both paving industries are represented in the area and have the capabilities to construct this project. The parameters identified in the 1993 AASHTO Guide—Appendix B does not clearly direct us toward a specific pavement type. This lack of a clear perspective regarding pavement type and the desire for competition is what caused the project team to believe this project might best be served by bidding alternate pavement type and therein giving both paving industries the opportunity to compete. The team ultimately concluded that alternative bidding of asphalt and concrete was best for this project.

Widening of Ramp A (I-471SB to I-275 WB)

During preliminary planning discussions on this project, the project team identified a need to widen the ramp that directs traffic from I-471 Southbound to I-275 Westbound. The department directed the design consultant to prepare a study on the feasibility of this proposed ramp widening. The results of the study can be found in Figure C. The existing ramp exits I-471 southbound in a free flow condition with one lane carrying approximately 1800 vehicles in the peak hour. The ramp joins I-275 with 400 feet of acceleration lane and an additional 300 feet of lane drop just prior to the bridge crossing over 3-Mile Road. The project team decided to evaluate widening Ramp A to two lanes for improved operation and additional capacity.

Three alternates were studied as potential solutions. Alternate 1 would widen Ramp A to two lanes and provide the recommended distance to drop the two lanes based on AASHTO criteria. In order to construct Alternate 1, Right of Way would have to be purchased, and the cost of such Right of Way is not reflected in the estimate below.

Alternate 2 is basically the same as Alternate 1 above but with the addition of a retaining wall. The retaining wall would be placed along the shoulder line of the Three Mile Road entrance ramp and serve as a barrier to eliminate the excavation and therefore keep construction inside the existing right of way.

Alternate 3 would widen Ramp A to two lanes and provide for 2160 feet of distance to drop the two lanes of I-275 leaving 350 feet of separation between the lane drop and the entrance ramp from Three Mile Road. This alternate would add lanes from Ramp A to the outside as in Alternate 1 and would require widening of the cut just east of Three Mile Road and extending the Three Mile Road Bridge. All of this work would be done within existing Right of Way. While this alternate does not provide the recommended distance for dropping the two lanes onto I-275 as recommended by AASHTO, the distance added appears to provide operational benefits with a reduced cost. A Corsim analysis found in Figure C was developed to look at the benefits of the Ramp widening.

| | <u>Estimated Cost</u> |
|--------------------------------------|------------------------------|
| Alternate #1 | \$4.4 Million |
| Alternate #2 (Alternate 1 with Wall) | \$5.3 Million |
| Alternate #3 | \$2.3 Million |

Based on discussions above, the conclusion of the Value Engineering team was to proceed with Alternate #3 for this project.

Maintenance of Traffic Schemes & Typical Section Widening

Several maintenance of traffic strategies were discussed during the value engineering study. There was general agreement among team members that the best traffic control plan would be to shift all traffic to lanes on the opposite travel way while construction was taking place. This would allow for construction to ensue unhampered by motor vehicles and would allow for improved traffic flow. The team felt that by reducing the number of contact points with construction entrances and reducing the “rubber necking” effect, construction would be expedited. However, given that there are five interchanges, three sets of twin bridges with open medians, and a raised planter type median with a 16 foot width through much of the project, it was realized that moving traffic to one side would be difficult without adding cost and time to the project.

The value engineering team settled on three traffic control schemes to analyze from an economic and timeliness perspective. The first alternate discussed was the traditional part width construction. This approach would maintain two lanes of traffic in both the north and southbound directions in the three and four lane sections for a majority of the project. In the existing 2 lane sections, traffic would be reduced to 1 lane. Since the existing outside concrete shoulders are only six inches in thickness, Phase 1 construction would involve strengthening the outside shoulders to carry traffic. Phase 2 would involve shifting traffic to the outside lane and newly constructed outside shoulder while the inside shoulder, inside lane, and half of the center lane was constructed. Phase 3 would shift traffic to the inside lane, and inside shoulder while the remaining section was constructed.

In examining the existing inside shoulders, it was discovered that dowel bar retrofits had been installed in isolated locations throughout the length of the project. Discussions with District maintenance personnel revealed that the retrofits were placed in the shoulders when mainline pavement repairs were done. These repairs required traffic to be shifted onto the shoulders. This caused concern among team members since it brought into question whether the inside shoulders would support traffic for short periods of time while the outside shoulder and lanes were constructed. The team settled on allowing traffic to use the inside 2 lanes and inside shoulder while constructing the outside shoulder in Phase 1. This would maintain the existing number of lanes in Phase 1 for the longest amount of time possible. If the inside shoulder began to fail in this Phase, then traffic would be prohibited from using the inside shoulder at that point, and the section would then be reduced by 1 lane. The total cost of this plan is estimated at 39.9 million. (a detailed cost analysis and typical section are illustrated in Figure D.) This estimate includes the replacement of all driving lanes and shoulders, the widening for Ramp A (I-471 southbound to I-275 westbound ramp) to two lanes, repair of damaged areas on the ramps in the four interchanges north of the I-275 interchange, and bridge maintenance work as previously discussed.

The second alternate discussed by the value engineering team would widen the typical section toward the median by eliminating the raised median and constructing an additional lane in each direction. Additional costs associated with this option include closing up the median on three sets of twin bridges, relocating the conventional light poles from the existing raised median to the new median barrier and relocating five overhead sign bases from the raised median to the

new median barrier. This alternate would assist in traffic control during construction but have an enhanced value of an additional lane in each direction for the majority of the project when construction was complete. The total cost of this plan is estimated at 56.1 million and includes all work described in alternate 1. (a detailed cost analysis and typical section are illustrated in Figure D.) This alternate would require design exceptions for reduced shoulder width, and ramp tapers lengths in a few locations along the project.

The third alternate discussed by the value engineering team involved the shifting of traffic to the opposite roadway direction and using moveable barrier wall to maintain three lanes of traffic in the direction of peak flow with two lanes of traffic in the non-peak flow direction. This alternate would require the removal of approximately 1000 feet of the raised median barrier at each crossover location, and would require the removal of several hundred feet of the median at any ramp access point provided. This alternate would also require the barrier wall to be moved twice a day for the duration of the project to accommodate peak flow conditions. Another concern of the team with this alternate was the existing lighting, overhead truss supports for signs, and median drainage structures all of which would have to be removed or relocated if this alternate were pursued. Due to the additional costs associated with alternate 3, and the additional time needed to prepare plan details, alternate 3 was considered undesirable by the value engineering team.

The team concluded that while widening the pavement and eliminating the planter boxes would assist in traffic control, there would still be significant amounts of time when traffic would be reduced to two lanes. In addition, the Transportation Cabinet is planning an extensive Public Involvement Plan for advising the public of lane restrictions and closure times associated with the project. The team recognized that the Cabinet would also be in communication with local governmental officials and agencies, major traffic generators, employers, etc. to advise of scheduled construction events, phasing times and dates, thereby assisting in traffic flow and reduced congestion.

The conclusion of the Value Engineering team was to proceed with Alternate #1.

Bridge Deck Overlays

The value engineering team discussed the existing bridge conditions and proposed rehabilitation strategies for bridges within the project limits. The bridges that will be receiving a deck treatment are as follows:

- I-471 SB over US 27
- I-471 NB over US 27
- I-471 SB over Grand Ave
- I-471 NB over Grand Ave
- I-471 SB over Chesapeake Ave
- I-471 NB over Chesapeake Ave
- I-471 NB Ramp to KY 8
- I-471 SB over KY 8
- I-471 SB over Ohio River
- I-471 NB over Ohio River

The team noted that the following bridge decks were in good condition and that no deck treatment would be required.

- I-471 SB over 6th St
- I-471 NB over 6th St
- I-471 NB over KY 8

The three alternates discussed for deck treatment were: 1) 6" Concrete Overlay 2) Epoxy Urethane Overlay 3) 1 ½" Latex Concrete Overlay.

The team agreed that the 6" Concrete Overlay alternate was not a good option on this project since it is only used when the deck is in very bad condition. The bridge decks throughout this project were judged to be in fair to good condition. In addition, since the pavement strategy is to remove the existing pavement to the bridge ends, there would be no need for a thick bridge deck overlay to minimize bridge end dig out length due to pavement tapers if it were not needed. Therefore the team eliminated the 6" concrete overlay option.

The Epoxy Urethane Overlay was the second alternate discussed by the team. This type of overlay is used as a water sealant and also serves to increase friction between the vehicle and riding surface. This overlay material is generally used when the bridge deck is in good shape and there are limited deck patches required. While the team thought this alternate may be viable on some of the bridges throughout the project, they were more comfortable with the concrete latex overlay option and felt that the latex option would last longer. Cost analysis shown in Figure E indicate that the concrete latex option and the epoxy urethane option were relatively similar in cost.

The Concrete Latex Overlay alternate was the final alternate discussed by the team. Cost considerations show it to be slightly less expensive than the other two options and the team felt that this option would have a much better performance period than the Epoxy Urethane option.

Thus Latex Concrete Overlay option was selected by the Value Engineering team. Figure E documents the cost of each alternate discussed.

SUMMARY AND CONCLUSIONS

The following areas were analyzed by the Value Engineering Team and from multiple discussions the following alternatives were developed and are recommended for implementation.

Recommendation #1: Pavement Design

The Value Engineering Team recommends that bidding alternate pavement types be implemented on this project. This strategy is believed to be best for the project by allowing both the asphalt and concrete industries to compete. In addition, a huge savings in initial cost should be recognized by bidding alternate pavements.

Recommendation #2: Widening of Ramp A (I-471SB to I-275 WB)

The Value Engineering Team recommends that Alternative Number 3 be implemented. This Alternate would accomplish the goals of the project team and is the least expensive alternate. This alternate would recognize a savings of 2.1 million dollars.

Recommendation #3: Maintenance of Traffic Schemes & Typical Section Widening

The Value Engineering Team recommends that Alternative Number 1 be implemented. This Alternate would accomplish the goals of the project team and is the least expensive alternate.

Recommendation #4: Bridge Deck Overlays

The Value Engineering Team recommends that Alternative Number 3 be implemented. This Alternate is the least expensive of the three alternates discussed, and the team believes this alternate to be superior to alternate 2 with respect to longevity. In addition, this alternate would recognize a cost savings of 286,000 dollars.

Figure A

Project Description: 6-2021.00, Campbell County LA71 MP 0.5-5 (100M Design)
NET PRESENT WORTH ANALYSIS

Date: 9/28/2011

Include Salvage Costs (Y/N) N
 Include User Costs (Y/N) N

| APPHALT OVERLAY | Thick (in) | Price (\$)/Unit | Alternate 1 15.75 in. Asphalt | Discount Rate | | | | | | | | | | | | | | | | |
|-----------------------------|------------|-----------------|----------------------------------|---------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|------------|---|------------|---|------------|---|
| | | | | Improvement Year | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | | | | | | |
| CL4 ASPH SURF 0.384 PG76-22 | 1.25 | 60.65 ton | | | | | | | | | | | | | | | | | | |
| CL4 ASPH BASE 1.00 PG76-22 | 3.0 | 48.15 ton | | | | | | | | | | | | | | | | | | |
| CL4 ASPH BASE 1.00 PG64-22 | 11.5 | 45.65 ton | | | | | | | | | | | | | | | | | | |
| CL4 ASPH BASE 1.00 PG64-22 | 0.0 | 45.65 ton | | | | | | | | | | | | | | | | | | |
| Total | 15.75 | | | | | | | | | | | | | | | | | | | |
| ASPHALT FULL-DEPTH | | | | | | | | | | | | | | | | | | | | |
| CL4 ASPH BASE 1.00 PG64-22 | 0.0 | 45.65 ton | | | | | | | | | | | | | | | | | | |
| CL4 ASPH BASE 1.00 PG64-22 | 4.0 | 38.00 ton | | | | | | | | | | | | | | | | | | |
| DRAINAGE BLANKET TYPE II | 4.0 | 20.00 ton | | | | | | | | | | | | | | | | | | |
| DGA | | | | | | | | | | | | | | | | | | | | |
| Asphalt Milling & Texturing | | 19.03 ton | | | | | | | | | | | | | | | | | | |
| Alt-1 Subtotal | | | 23,268,779 | 0 | 21,988,267 | 0 | 21,176,559 | 0 | 20,845,418 | 0 | 20,287,009 | 0 | 20,038,014 | 0 | 20,038,014 | 0 | 20,038,014 | 0 | 20,038,014 | 0 |
| Alt-1 Total NPV | | | 23,268,779 | 0 | 21,988,267 | 0 | 21,176,559 | 0 | 20,845,418 | 0 | 20,287,009 | 0 | 20,038,014 | 0 | 20,038,014 | 0 | 20,038,014 | 0 | 20,038,014 | 0 |

| PCC OVERLAY | Thick (in) | Price (\$)/Unit | Alternate 2 13 in. PCC | Discount Rate | | | | | | | | | | | | | | | | |
|---|------------|-----------------|---------------------------|---------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|---------------------|-------------------|------------|-----------|------------|-----------|------------|-----------|
| | | | | Improvement Year | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | Agency Cost (\$) | User Cost (\$) | | | | | | |
| PCC PAVEMENT | 13.0 | 53.00 sq yd | | | | | | | | | | | | | | | | | | |
| PCC SHOULDER | 13.0 | 49.50 sq yd | | | | | | | | | | | | | | | | | | |
| PCC FULL-DEPTH | 0 | 0.00 ton | | | | | | | | | | | | | | | | | | |
| PCC BASE | 0.0 | 0.00 sq yd | | | | | | | | | | | | | | | | | | |
| PCC Drainage Blanket | 0.0 | 0.00 ton | | | | | | | | | | | | | | | | | | |
| DGA | 4.0 | 20.00 ton | | | | | | | | | | | | | | | | | | |
| Clean and Seal Trans. Joints | 1.15 in ft | | | | | | | | | | | | | | | | | | | |
| Clean and Seal Long Joints | 1.15 in ft | | | | | | | | | | | | | | | | | | | |
| Diamond Grinding | 3 sq yd | | | | | | | | | | | | | | | | | | | |
| Alt-2 Subtotal | | | 21,730,441 | 0 | 20,746,704 | 0 | 18,898,092 | 0 | 18,295,517 | 0 | 18,023,708 | 0 | 17,982,510 | 0 | 17,982,510 | 0 | 17,982,510 | 0 | 17,982,510 | 0 |
| Alt-2 Total NPV | | | 21,730,441 | 0 | 20,746,704 | 0 | 18,898,092 | 0 | 18,295,517 | 0 | 18,023,708 | 0 | 17,982,510 | 0 | 17,982,510 | 0 | 17,982,510 | 0 | 17,982,510 | 0 |
| Initial Construction Alt 2 | | | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 | 17,395,605 | 5,439,495 |
| PCC Repair & Diamond Grinding | 2027 | 1,874,695 | 8,388,204 | 1,392,926 | 6,232,618 | 1,040,951 | 4,657,716 | 782,245 | 3,500,138 | 590,982 | 2,844,337 | 448,787 | 2,008,088 | 18,117 | | | | | | |
| PCC Repair & Diamond Grinding | 2042 | 2,460,141 | | 1,358,172 | | 461,535 | | 117,667 | | 37,121 | | | | | | | | | | |
| FALSE | | | | | | | | | | | | | | | | | | | | |
| Salvage | | | 2052 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| LOCA Cost Difference (with User Costs) = | | | -7.08% | | | | | | | | | | | | | | | | | |
| LCCA Cost Difference (without User Costs) = | | | -7.08% | | | | | | | | | | | | | | | | | |
| Asph Rehab Costs = | | | 4,024,197 | 18,231,206 | 2,743,685 | 12,978,609 | 1,931,976 | 9,375,986 | 1,400,836 | 6,872,091 | 1,042,427 | 5,108,885 | 793,431 | 3,850,801 | | | | | | |
| PCC Rehab Costs = | | | 4,394,836 | 8,388,284 | 2,751,098 | 6,232,618 | 1,502,487 | 4,657,716 | 899,912 | 3,500,138 | 628,103 | 2,644,337 | 466,904 | 2,008,088 | | | | | | |
| Rehab Cost Difference = | | | -310,639 | -7.413 | -429,490 | -12.06% | -12.06% | -12.84% | -12.84% | -12.56% | -12.56% | -12.18% | -12.18% | | | | | | | |

NOTE: Asphalt prices are from I-71 Gallatin/Boone and JPC Prices are from Campbell Co. I-275
 NOTE: Night work only assumed to begin in 2035

Figure B

**SUMMARY OF OVERLAY VS FULL DEPTH ESTIMATES
1-471 RAMPS PAVEMENT REHABILITATION
CAMPBELL COUNTY
ITEM NO. 6-2021.00**

| COST | FULL DEPTH REPLACEMENT | JPC OVERLAY |
|--|-------------------------------|---------------------|
| 3 LANE SECTION | \$7,894,709 | \$7,563,924 |
| 4 LANE SECTION | \$5,623,775 | \$5,004,640 |
| BIFURCATED SECTION | \$2,615,657 | \$1,728,327 |
| RAMPS | \$0 | \$576,893 |
| TOTAL ESTIMATED PROJECT PAVING COSTS | \$16,134,141 | \$14,873,783 |
| TOTAL ESTIMATED COST DIFFERENCE FULL DEPTH VS OVERLAY | \$1,260,357 | |

ONLY AREAS WHERE A JPC OVERLAY WOULD BE AN OPTION WERE INCLUDED IN THIS REPORT.

OVERLAY VS FULL DEPTH ESTIMATES
 4 LANE SECTION
 1-471 PAVEMENT REHABILITATION
 CAMPBELL COUNTY
 ITEM NO. 6-2021.00

| CODE | ITEM | UNIT | QUANTITY - FULL DEPTH OPTION PER FOOT OF ROADWAY | COST/UNIT | TOTAL COST FULL DEPTH | QUANTITY - OVERLAY OPTION PER FOOT OF ROADWAY | COST/UNIT | TOTAL COST OVERLAY |
|------------|---|--------|---|-----------|------------------------|---|-----------|------------------------|
| 1 | DGA | TON | | | | 0.17 | \$ 25.00 | \$ 4.32 |
| 3 | CRUSHED STONE BASE | TON | 0.84 | \$ 25.00 | \$ 20.97 | | | |
| 8 | CEMENT STABILIZED ROADBED | SQ YD | 7.56 | \$ 6.00 | \$ 45.33 | | | |
| 72 | CRUSHED AGGREGATE SIZE NO. 57 | TON | | | | 0.18 | \$ 31.40 | \$ 5.63 |
| 269 | MODIFIED OPEN-GRADED DRAINAGE COURSE | TON | | | | 0.06 | \$ 80.88 | \$ 4.45 |
| 358 | ASPHALT CURING SEAL | TON | 0.0076 | \$ 632.00 | \$ 4.78 | | | |
| 1615 | CONC MED BARR BOX INLET TY 14B2 1) | LF | | | | 1 | \$ 6.98 | \$ 6.98 |
| 2058 | REMOVE PCC PAVEMENT | SQ YD | 7.56 | \$ 11.00 | \$ 83.11 | 0.22 | \$ 11.00 | \$ 2.44 |
| 2069 | JPC PAVEMENT - 10 IN | SQ YD | | | | 8.22 | \$ 50.00 | \$ 411.11 |
| 2086 | JPC PAVEMENT - 13 IN | SQ YD | 7.56 | \$ 53.00 | \$ 400.44 | | | |
| 2200 | ROADWAY EXCAVATION | CU YD | 1.68 | \$ 15.00 | \$ 25.19 | 0.09 | \$ 15.00 | \$ 1.34 |
| 2542 | CEMENT | TON | 0.15 | \$ 110.00 | \$ 16.25 | | | |
| 2702 | SAND FOR BLOTTER | TON | 0.02 | \$ 30.00 | \$ 0.57 | | | |
| 21935EN | REMOVE CONC MEDIAN BARRIER | LF | | | | 1 | \$ 30.00 | \$ 30.00 |
| 23045ES508 | CONCRETE MEDIAN BARRIER TY 14B(50) | LF | | | | 1 | \$ 45.00 | \$ 45.00 |
| | | | TOTAL PER FOOT OF ROADWAY | | \$ 596.63 | | | \$ 511.28 |
| | | | LENGTH OF 4 LANE SECTION | | 8,200 | | | 8,200 |
| | | | LENGTH FOR BARRIER WALL REPLACEMENT ITEMS REQUIRED IN FULL DEPTH TAPERS | | | | | 2,000 |
| | | | | SUBTOTAL | \$ 4,892,366.00 | | | \$ 4,353,753.48 |
| 2568 | MOBILIZATION | LP SUM | | | \$ 146,770.98 | | | \$ 130,612.60 |
| 2569 | DEMobilIZATION | LP SUM | | | \$ 73,385.49 | | | \$ 65,306.30 |
| | TOTAL ESTIMATED CONSTRUCTION COSTS | | | | \$ 5,112,522.47 | | | \$ 4,549,672.39 |
| | 10% ENGINEERING & CONTINGENCIES | | | | \$ 511,252.25 | | | \$ 454,967.24 |
| | TOTAL ESTIMATED 4 LANE SECTION COSTS | | | | \$ 5,623,774.72 | | | \$ 5,004,639.63 |

1) 11 TOTAL BOX INLETS @ \$5,200 EA / LENGTH OF OVERLAY

OVERLAY VS FULL DEPTH ESTIMATES
 3 LANE SECTIONS
 1-471 PAVEMENT REHABILITATION
 CAMPBELL COUNTY
 ITEM NO. 6-2021.00

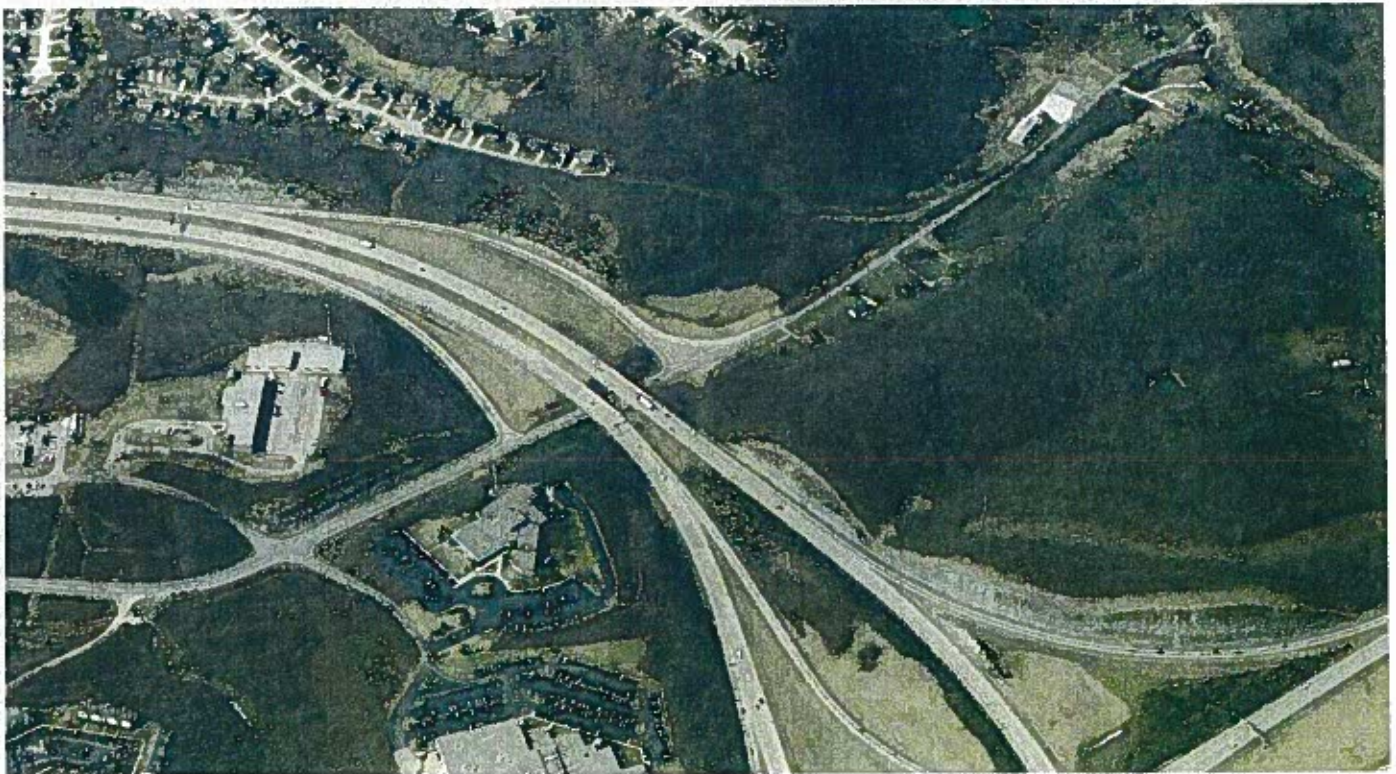
| CODE | ITEM | UNIT | QUANTITY - FULL DEPTH OPTION PER FOOT OF ROADWAY | COST/UNIT | TOTAL COST FULL DEPTH | QUANTITY - OVERLAY OPTION PER FOOT OF ROADWAY | COST/UNIT | TOTAL COST OVERLAY |
|------------|---|--------|---|-----------|------------------------|---|-----------|------------------------|
| 1 | DGA | TON | | | | 0.17 | \$ 25.00 | \$ 4.32 |
| 3 | CRUSHED STONE BASE | TON | 0.69 | \$ 25.00 | \$ 17.27 | | | |
| 8 | CEMENT STABILIZED ROADBED | SQ YD | 6.22 | \$ 6.00 | \$ 37.33 | | | |
| 72 | CRUSHED AGGREGATE SIZE NO. 57 | TON | | | | 0.18 | \$ 31.40 | \$ 5.63 |
| 269 | MODIFIED OPEN-GRADED DRAINAGE COURSE | TON | | | | 0.05 | \$ 80.88 | \$ 3.71 |
| 358 | ASPHALT CURING SEAL | TON | 0.0062 | \$ 632.00 | \$ 3.93 | | | |
| 1615 | CONC MED BARR BOX INLET TY 14B2 1) | LF | | | | 1 | \$ 10.79 | \$ 10.79 |
| 2058 | REMOVE PCC PAVEMENT | SQ YD | 6.22 | \$ 11.00 | \$ 68.44 | 0.22 | \$ 11.00 | \$ 2.44 |
| 2069 | JPC PAVEMENT - 10 IN | SQ YD | 6.22 | \$ 53.00 | \$ 329.78 | 6.89 | \$ 50.00 | \$ 344.44 |
| 2086 | JPC PAVEMENT - 13 IN | SQ YD | 6.22 | \$ 53.00 | \$ 329.78 | | | |
| 2200 | ROADWAY EXCAVATION | CU YD | 1.38 | \$ 15.00 | \$ 20.74 | 0.09 | \$ 15.00 | \$ 1.34 |
| 2542 | CEMENT | TON | 0.12 | \$ 110.00 | \$ 13.38 | | | |
| 2702 | SAND FOR BLOTTER | TON | 0.02 | \$ 30.00 | \$ 0.47 | | | |
| 21935EN | REMOVE CONC MEDIAN BARRIER | LF | | | | 1 | \$ 30.00 | \$ 30.00 |
| 23045ES508 | CONCRETE MEDIAN BARRIER TY 14B(50) | LF | | | | 1 | \$ 45.00 | \$ 45.00 |
| | | | TOTAL PER FOOT OF ROADWAY | | \$ 491.34 | | | \$ 447.68 |
| | | | LENGTH OF 3 LANE SECTIONS | | 13,978 | | | 13,978 |
| | | | LENGTH FOR BARRIER WALL REPLACEMENT ITEMS REQUIRED IN FULL DEPTH TAPERS | | | | | 4,000 |
| | | | SUBTOTAL | | \$ 6,867,950.52 | | | \$ 6,580,186.00 |
| 2568 | MOBILIZATION | LP SUM | | | \$ 206,038.52 | | | \$ 197,405.58 |
| 2569 | DEMobilIZATION | LP SUM | | | \$ 103,019.26 | | | \$ 98,702.79 |
| | TOTAL ESTIMATED CONSTRUCTION COSTS | | | | \$ 7,177,008.29 | | | \$ 6,876,294.37 |
| | 10% ENGINEERING & CONTINGENCIES | | | | \$ 717,700.83 | | | \$ 687,629.44 |
| | TOTAL ESTIMATED 3 LANE SECTION COSTS | | | | \$ 7,894,709.12 | | | \$ 7,563,923.81 |

1) 29 TOTAL BOX INLETS @ \$5,200 EA / LENGTH OF OVERLAY

Figure C



**EVALUATION OF TWO-LANE RAMP WIDENING
ON RAMP A FROM I-471 SB TO I-275 WB
IN CAMPBELL COUNTY**



PREPARED BY:
WMB INC.
James Napier, PE

PREPARED FOR:
KYTC CENTRAL OFFICE DESIGN
DAN HITE, PE



FINAL

July 15, 2011



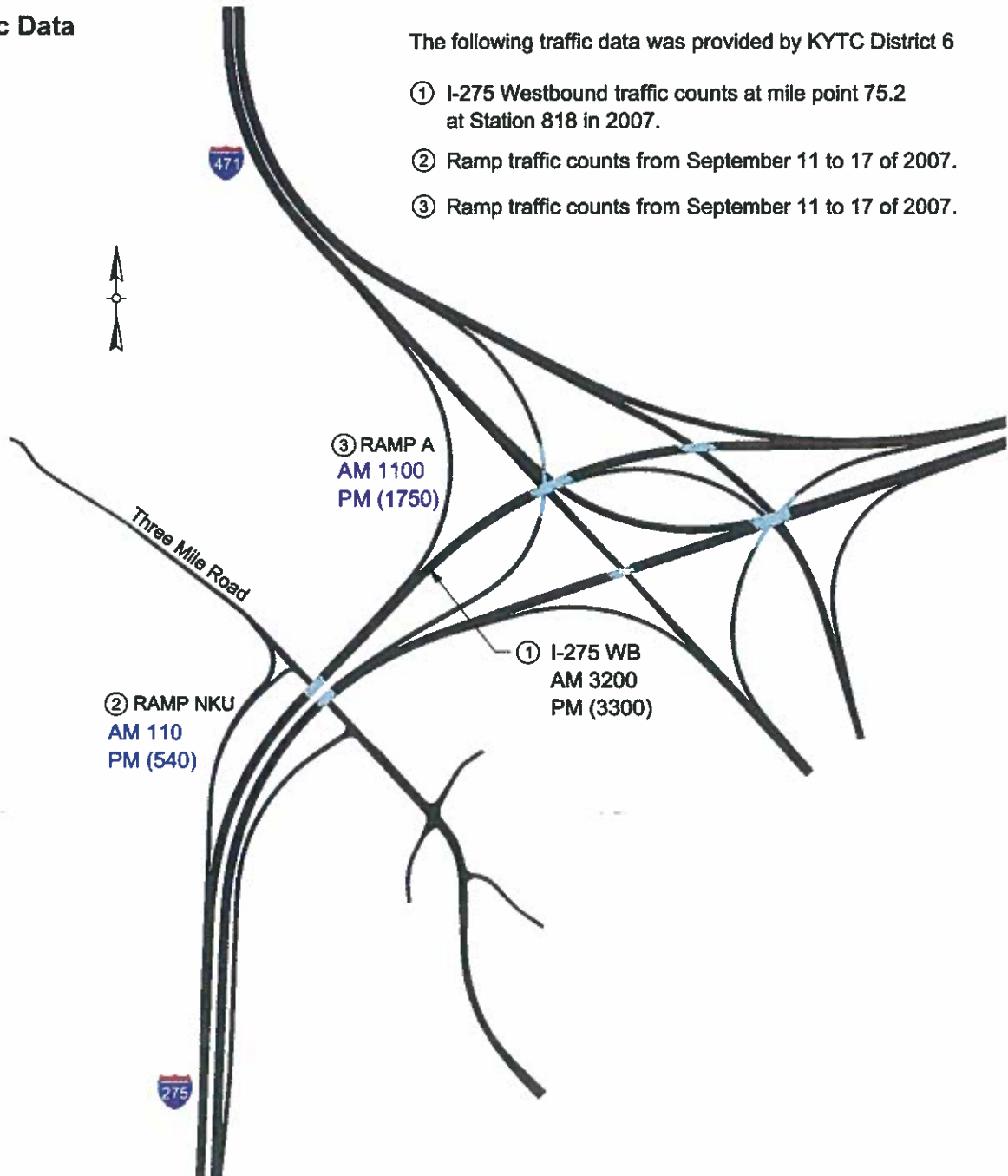
EVALUATION OF TWO LANE RAMP WIDENING ON RAMP A FROM I-471 SB TO I-275 WB IN CAMPBELL COUNTY

The following study was prepared on the proposed widening of Ramp A from I-471 Southbound to I-275 Westbound in Campbell County. The existing ramp exits I-471 Southbound in a free flow condition with one lane that carries approximately 1800 vehicles in the peak hour to I-275. The ramp joins I-275 Westbound with 400' of acceleration lane and a 300' lane drop just prior to the bridge crossing over 3-mile Road. Given the short acceleration distance and current traffic volumes on Ramp A and I-275 Westbound, the project team decided to evaluate widening Ramp A to two lanes for additional capacity and improved operation.

Traffic Data

The following traffic data was provided by KYTC District 6

- ① I-275 Westbound traffic counts at mile point 75.2 at Station 818 in 2007.
- ② Ramp traffic counts from September 11 to 17 of 2007.
- ③ Ramp traffic counts from September 11 to 17 of 2007.



Alternate 1

Alternate 1 would widen Ramp A to two lanes and provide 2500 feet of distance to drop the two lanes on I-275 based on the recommended distance in AASHTO "A Policy on Geometric Design of Highway and Streets 2004". This configuration would overlap the entrance ramp from Three Mile Road and result in reconstructing the Three Mile road ramp approximately 800ft to the west. The additional lanes along I-275 for Ramp A were added to the outside for this alternate which results in widening the cuts and fills along I-275 and the Westbound bridge over Three Mile Road. Shifting the entrance ramp and taper from Three Mile Road requires significant excavation and is likely to extend outside the existing right of way. An option is included with Alternate 1 that uses a retaining wall along the shoulder line of the Three Mile Road entrance ramp to eliminate the excavation and keep the construction inside the existing right of way. The approximate construction costs are as follows with an itemized breakdown on Page 4.

| | | |
|---------------------|---|-------------|
| Alternate 1 | - | \$4,387,770 |
| Alternate 1 w/ Wall | - | \$5,282,130 |



Alternate 2

Alternate 2 would widen Ramp A to two lanes and provide 2160 feet of distance to drop the two lanes on I-275 which would leave 350 feet of separation between the lane drop and the entrance ramp from Three Mile Road. This alternate would add the lanes from Ramp A to the outside as in Alternate 1 which requires widening the cut just east of Three Mile Road and the bridge, but is expected to be constructed within the existing right of way. While this alternate does not provide the recommended distance for dropping the two lanes onto I-275 as mentioned with Alternate 1, the distance provided does appear to provide operational benefits with reduced cost.

| | | |
|-------------|---|-------------|
| Alternate 2 | - | \$2,252,670 |
|-------------|---|-------------|

Note:

PDFs are included with this study that show plan, profile, and cross sections for the alternates described here.

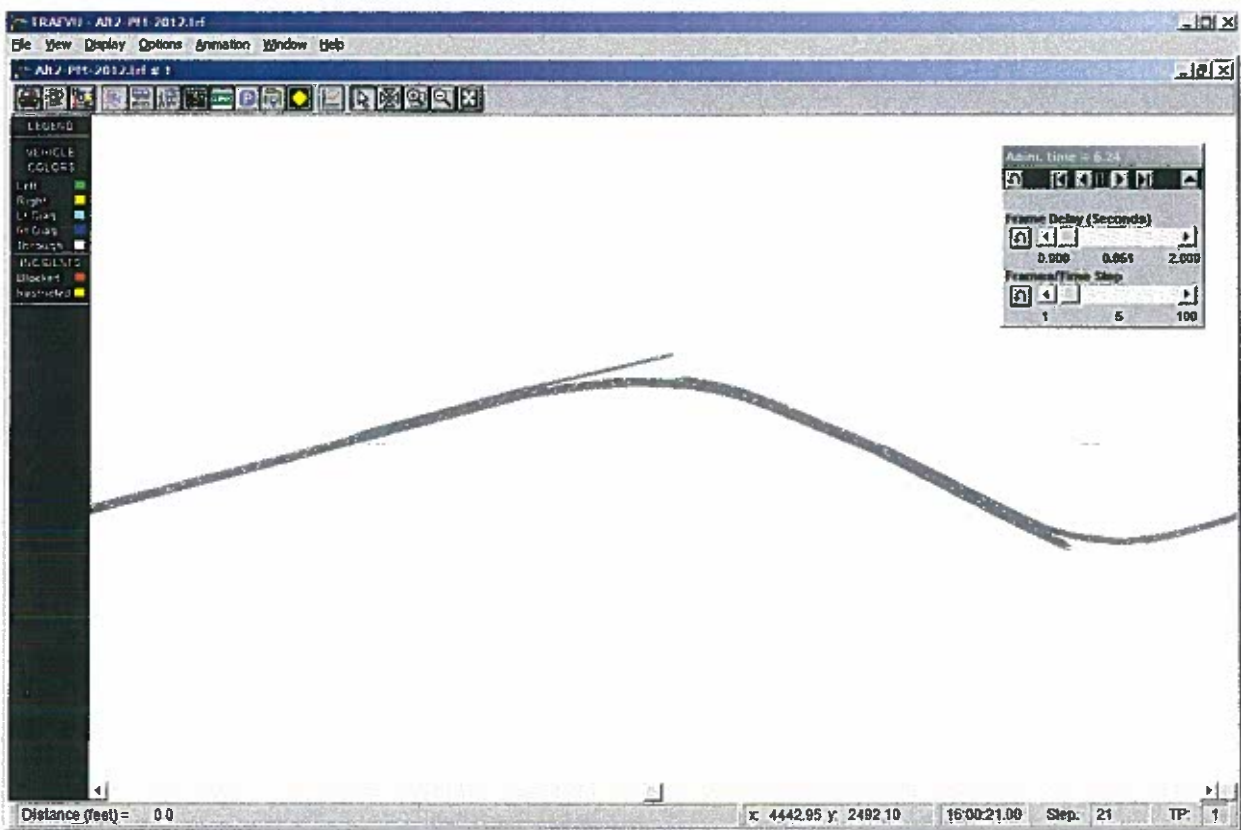
Traffic Analysis

A Corsim network was developed for the project area using the PM peak traffic counts shown earlier and increasing to 2012 and 2022 volumes using a 2% growth rate. Traffic simulations were run for the existing condition, Alternate 1 and Alternate 2. The network wide results below are based on an average of ten one hour simulations for each condition.

| PM DHV | I-275 WB | From I-471 | From Three Mile Road |
|--------|----------|------------|----------------------|
| 2007 | 3300 | 1750 | 540 |
| 2012 | 3650 | 1950 | 590 |
| 2022 | 4450 | 2370 | 690 |

| Analysis Year | Existing Condition | | | Alternate 1 | | | Alternate 2 | | |
|---------------|--------------------|------------------------|----------------------|-----------------|------------------------|----------------------|-----------------|------------------------|----------------------|
| | Avg Speed (mph) | Delay Travel (veh-hrs) | Total Travel (hours) | Avg Speed (mph) | Delay Travel (veh-hrs) | Total Travel (hours) | Avg Speed (mph) | Delay Travel (veh-hrs) | Total Travel (hours) |
| 2012 | 56.18 | 10.70 | 114.94 | 58.07 | 6.51 | 111.20 | 57.73 | 7.79 | 111.86 |
| 2022 | 46.00 | 41.74 | 168.28 | 53.09 | 20.30 | 147.78 | 50.00 | 30.12 | 156.98 |

Snapshot of Corsim network



Alternate 1

| ITEM | QUANTITY | UNIT | UNIT PRICE | AMOUNT |
|-----------------------|----------|------|--------------|----------------|
| Excavation | 155000 | CY | \$10.00 | \$1,550,000.00 |
| Barrier Wall | 1010 | LF | \$20.00 | \$20,200.00 |
| Crash Cushion | 1 | EA | \$5,000.00 | \$5,000.00 |
| 13" Concrete Pavement | 23500 | SY | \$45.00 | \$1,057,500.00 |
| DGA | 16600 | TON | \$16.50 | \$273,900.00 |
| Drainage Blanket | 2500 | TON | \$37.00 | \$92,500.00 |
| Bridge Widening | 3185 | SF | \$175.00 | \$557,375.00 |
| Signing | 1 | LS | \$100,000.00 | \$100,000.00 |
| Misc | 1 | LS | \$731,295.00 | \$731,295.00 |
| | | | | \$4,387,770.00 |

Alternate 1 With Retaining Wall

| ITEM | QUANTITY | UNIT | UNIT PRICE | AMOUNT |
|-----------------------|----------|------|--------------|----------------|
| Excavation | 52000 | CY | \$10.00 | \$520,000.00 |
| Barrier Wall | 1010 | LF | \$20.00 | \$20,200.00 |
| Retaining Wall | 17400 | SF | \$100.00 | \$1,740,000.00 |
| Crash Cushion | 2 | EA | \$5,000.00 | \$10,000.00 |
| 13" Concrete Pavement | 24100 | SY | \$45.00 | \$1,084,500.00 |
| DGA | 16800 | TON | \$16.50 | \$277,200.00 |
| Drainage Blanket | 2500 | TON | \$37.00 | \$92,500.00 |
| Bridge Widening | 3185 | SF | \$175.00 | \$557,375.00 |
| Signing | 1 | LS | \$100,000.00 | \$100,000.00 |
| Misc | 1 | LS | \$880,355.00 | \$880,355.00 |
| | | | | \$5,282,130.00 |

Alternate 2

| ITEM | QUANTITY | UNIT | UNIT PRICE | AMOUNT |
|-----------------------|----------|------|--------------|----------------|
| Excavation | 45000 | CY | \$10.00 | \$450,000.00 |
| 13" Concrete Pavement | 13000 | SY | \$45.00 | \$585,000.00 |
| DGA | 8400 | TON | \$16.50 | \$138,600.00 |
| Drainage Blanket | 1250 | TON | \$37.00 | \$46,250.00 |
| Bridge Widening | 3185 | SF | \$175.00 | \$557,375.00 |
| Signing | 1 | LS | \$100,000.00 | \$100,000.00 |
| Misc | 1 | LS | \$375,445.00 | \$375,445.00 |
| | | | | \$2,252,670.00 |

Figure D

PRE-DESIGN ESTIMATES
1-471 PAVEMENT REHABILITATION
CAMPBELL COUNTY
ITEM NO. 6-2021.00

| CODE | ITEM | UNIT | FULL DEPTH REPLACEMENT EXIST LANES & OUTSIDE SHLD | COST/UNIT | TOTAL COST | FULL DEPTH REPLACEMENT ENTIRE ROADWAY & ADD TWO LANES IN MEDIAN | COST/UNIT | TOTAL COST |
|------|--|--------|---|-----------------|------------------|---|-----------------|------------------|
| 1 | DGA BASE | TON | 68,981 | \$ 18.00 | \$ 1,241,658.00 | 85,517 | \$ 18.00 | \$ 1,539,306.00 |
| 8 | CEMENT STABILIZED ROADBED | SQ YD | 299,919 | \$ 2.00 | \$ 599,838.00 | 371,813 | \$ 2.00 | \$ 743,626.00 |
| 19 | DRAINAGE BLANKET TYPE III-CEM | SQ YD | 299,919 | \$ 10.00 | \$ 2,999,190.00 | 371,813 | \$ 10.00 | \$ 3,718,130.00 |
| 100 | ASPHALT SEAL AGGREGATE | TON | 582 | \$ 42.00 | \$ 24,460.80 | 565 | \$ 42.00 | \$ 23,742.60 |
| 281 | EMULSIFIED ASPHALT RS-2 | TON | 69.9 | \$ 651.00 | \$ 45,504.90 | 67.8 | \$ 651.00 | \$ 44,137.80 |
| 358 | ASPHALT CURING SEAL | TON | 299.9 | \$ 632.00 | \$ 189,536.80 | 371.8 | \$ 632.00 | \$ 234,977.60 |
| 521 | STORM SEWER PIPE-15 IN | LF | | | | 450 | \$ 50.00 | \$ 22,500.00 |
| 1001 | PERFORATED PIPE-6 IN | LF | 103,355 | \$ 5.40 | \$ 558,117.00 | 81,565 | \$ 5.40 | \$ 440,451.00 |
| 1015 | INSPECT AND CERTIFY EDGE DRAIN SYSTEM | LP SUM | 1 | \$ 15,000.00 | \$ 15,000.00 | 1 | \$ 15,000.00 | \$ 15,000.00 |
| 1021 | PERF PIPE HEADWALL TY 1-6 IN | EACH | 104 | \$ 516.00 | \$ 53,664.00 | 100 | \$ 516.00 | \$ 51,600.00 |
| 1615 | CONC MED BARR BOX INLET TY 14B2 | EACH | | | | 45 | \$ 5,200.00 | \$ 234,000.00 |
| 1982 | DELINEATOR FOR GUARDRAIL-WHITE | EACH | 294 | \$ 8.50 | \$ 2,499.00 | 294 | \$ 8.50 | \$ 2,499.00 |
| 1984 | DELINEATOR FOR BARRIER-WHITE | EACH | 61 | \$ 7.55 | \$ 458.41 | 61 | \$ 7.55 | \$ 458.41 |
| 1985 | DELINEATOR FOR BARRIER-YELLOW | EACH | 61 | \$ 7.55 | \$ 458.41 | 61 | \$ 7.55 | \$ 458.41 |
| 2003 | RELOCATE TEMP CONC BARRIER | LF | 55,833 | \$ 4.40 | \$ 245,665.20 | 55,833 | \$ 4.40 | \$ 245,665.20 |
| 2014 | BARRICADE-TYPE III | EACH | 25 | \$ 83.00 | \$ 2,075.00 | 25 | \$ 83.00 | \$ 2,075.00 |
| 2058 | REMOVE PCC PAVEMENT | SQ YD | 302,611 | \$ 15.00 | \$ 4,539,165.00 | 341,242 | \$ 15.00 | \$ 5,118,630.00 |
| 2086 | JPC PAVEMENT - 13 IN | SQ YD | 304,082 | \$ 42.00 | \$ 12,771,444.00 | 371,813 | \$ 42.00 | \$ 15,616,146.00 |
| 2200 | ROADWAY EXCAVATION | CU YD | 33,325 | \$ 15.00 | \$ 499,875.00 | 52,230 | \$ 15.00 | \$ 783,450.00 |
| 2237 | DITCHING | LF | 10,000 | \$ 2.00 | \$ 20,000.00 | 10,000 | \$ 2.00 | \$ 20,000.00 |
| 2363 | GUARDRAIL CONNECTOR TO BRIDGE END TY A | EACH | 10 | \$ 2,222.00 | \$ 22,220.00 | 10 | \$ 2,222.00 | \$ 22,220.00 |
| 2367 | GUARDRAIL END TREATMENT TYPE 1 | EACH | 4 | \$ 2,288.00 | \$ 9,152.00 | 4 | \$ 2,288.00 | \$ 9,152.00 |
| 2369 | GUARDRAIL END TREATMENT TYPE 2A | EACH | 8 | \$ 696.00 | \$ 5,568.00 | 8 | \$ 696.00 | \$ 5,568.00 |
| 2373 | GUARDRAIL END TREATMENT TYPE 3 | EACH | 2 | \$ 727.00 | \$ 1,454.00 | 2 | \$ 727.00 | \$ 1,454.00 |
| 2381 | REMOVE GUARDRAIL | LF | 21,188 | \$ 1.30 | \$ 27,543.75 | 21,188 | \$ 1.30 | \$ 27,543.75 |
| 2387 | GUARDRAIL CONNECTOR TO BRIDGE END TY A-1 | EACH | 7 | \$ 430.00 | \$ 3,010.00 | 7 | \$ 430.00 | \$ 3,010.00 |
| 2391 | GUARDRAIL END TREATMENT TYPE 4A | EACH | 7 | \$ 1,984.00 | \$ 13,888.00 | 7 | \$ 1,984.00 | \$ 13,888.00 |
| 2542 | CEMENT | TON | 5,863 | \$ 110.00 | \$ 644,974.00 | 7,269 | \$ 110.00 | \$ 799,634.00 |
| 2562 | SIGNS | SQ FT | 3,000 | \$ 4.59 | \$ 13,770.00 | 3,000 | \$ 4.59 | \$ 13,770.00 |
| 2650 | MAINTAIN & CONTROL TRAFFIC | LP SUM | 1 | \$ 1,500,000.00 | \$ 1,500,000.00 | 1 | \$ 2,000,000.00 | \$ 2,000,000.00 |
| 2671 | PORTABLE CHANGEABLE MESSAGE SIGN | EACH | 20 | \$ 3,200.00 | \$ 64,000.00 | 20 | \$ 3,200.00 | \$ 64,000.00 |
| 2701 | TEMP SILT FENCE | LF | 25,000 | \$ 2.00 | \$ 50,000.00 | 25,000 | \$ 2.00 | \$ 50,000.00 |
| 2702 | SAND FOR BLOTTER | TON | 750 | \$ 30.00 | \$ 22,500.00 | 930 | \$ 30.00 | \$ 27,900.00 |
| 2705 | SILT TRAP TYPE C | EACH | 200 | \$ 131.00 | \$ 26,200.00 | 200 | \$ 131.00 | \$ 26,200.00 |
| 2708 | CLEAN SILT TRAP TYPE C | EACH | 200 | \$ 29.00 | \$ 5,800.00 | 200 | \$ 29.00 | \$ 5,800.00 |
| 2709 | CLEAN TEMP SILT FENCE | LF | 25,000 | \$ 0.67 | \$ 16,750.00 | 25,000 | \$ 0.67 | \$ 16,750.00 |
| 2726 | STAKING | LP SUM | 1 | \$ 50,000.00 | \$ 50,000.00 | 1 | \$ 150,000.00 | \$ 150,000.00 |
| 2775 | ARROW PANEL | EACH | 5 | \$ 900.00 | \$ 4,500.00 | 5 | \$ 900.00 | \$ 4,500.00 |
| 2894 | CRASH CUSHION TYPE VI-T | EACH | 10 | \$ 5,000.00 | \$ 50,000.00 | 10 | \$ 5,000.00 | \$ 50,000.00 |
| 3171 | CONCRETE BARRIER WALL TYPE 9T | LN FT | 55,833 | \$ 20.70 | \$ 1,155,743.10 | 55,833 | \$ 20.70 | \$ 1,155,743.10 |
| 5950 | EROSION CONTROL BLANKET | SQ YD | 50,000 | \$ 0.97 | \$ 48,500.00 | 50,000 | \$ 0.97 | \$ 48,500.00 |

**PRE-DESIGN ESTIMATES
1-471 PAVEMENT REHABILITATION
CAMPBELL COUNTY
ITEM NO. 6-2021.00**

| CODE | ITEM | UNIT | FULL DEPTH REPLACEMENT EXIST LANES & OUTSIDE SHLD | COST/UNIT | TOTAL COST | FULL DEPTH REPLACEMENT ENTIRE ROADWAY & ADD TWO LANES IN MEDIAN | COST/UNIT | TOTAL COST |
|-----------|---|--------|---|-----------------|-------------------------|---|-----------------|-------------------------|
| 5985 | SEEDING AND PROTECTION | SOYD | 50,000 | \$ 0.35 | \$ 17,500.00 | 50,000 | \$ 0.35 | \$ 17,500.00 |
| 6417 | FLEXIBLE DELINEATOR POST-W | EACH | 161 | \$ 30.00 | \$ 4,816.24 | 161 | \$ 30.00 | \$ 4,816.24 |
| 6418 | FLEXIBLE DELINEATOR POST-Y | EACH | 69 | \$ 30.00 | \$ 2,076.89 | 69 | \$ 30.00 | \$ 2,076.89 |
| 6511 | PAVE STRIPING-TEMP PAINT-6 IN | LF | 200,000 | \$ 0.20 | \$ 40,000.00 | 200,000 | \$ 0.20 | \$ 40,000.00 |
| 6549 | PAVE STRIPING-TEMP REM TAPE-B | LF | 20,000 | \$ 2.28 | \$ 45,600.00 | 20,000 | \$ 2.28 | \$ 45,600.00 |
| 6550 | PAVE STRIPING-TEMP REM TAPE-W | LF | 20,000 | \$ 1.43 | \$ 28,600.00 | 20,000 | \$ 1.43 | \$ 28,600.00 |
| 6551 | PAVE STRIPING-TEMP REM TAPE-Y | LF | 20,000 | \$ 1.54 | \$ 30,800.00 | 20,000 | \$ 1.54 | \$ 30,800.00 |
| 6592 | PAVEMENT MARKER TYPE V-B W/R | EACH | 1,050 | \$ 22.00 | \$ 23,091.93 | 1,050 | \$ 22.00 | \$ 23,091.93 |
| 6585 | PAVEMENT MARKER TYPE IVA-MW TEMP | EACH | 2,000 | \$ 6.95 | \$ 13,900.00 | 2,000 | \$ 6.95 | \$ 13,900.00 |
| 10020NS | FUEL ADJUSTMENT | DOLL | 1 | \$ 67,224.00 | \$ 67,224.00 | 1 | \$ 82,568.00 | \$ 82,568.00 |
| 21802EN | G/R STEEL W BEAM-S FACE (7 FT POST) | LF | 21,187.5 | \$ 16.00 | \$ 339,000.00 | 21,187.5 | \$ 16.00 | \$ 339,000.00 |
| 21935EN | REMOVE CONC MEDIAN BARRIER | LF | | | | 33,641 | \$ 30.00 | \$ 1,009,230.00 |
| 22854EN | PAVE STRIPE PERM-6 IN HD21-WHITE | LF | 81,202 | \$ 0.24 | \$ 19,488.48 | 91,851 | \$ 0.24 | \$ 22,044.26 |
| 22855EN | PAVE STRIPE PERM-6 IN HD21-YELLOW | LF | 56,442 | \$ 0.24 | \$ 13,546.15 | 56,442 | \$ 0.24 | \$ 13,546.15 |
| 22856EN | PAVE STRIPE PERM-12 IN HD21-WHITE | LF | 10,500 | \$ 0.48 | \$ 5,040.00 | 10,500 | \$ 0.48 | \$ 5,040.00 |
| 23045ES08 | CONCRETE MEDIAN BARRIER TY 14B(50) | LF | | | | 17,958 | \$ 45.00 | \$ 808,110.00 |
| | RAMP REPAIRS (1) | LP SUM | 1 | \$ 2,000,000.00 | \$ 2,000,000.00 | 1 | \$ 2,000,000.00 | \$ 2,000,000.00 |
| | BRIDGE MAINTENANCE REPAIR COSTS (2) | LP SUM | 1 | \$ 4,525,070.00 | \$ 4,525,070.00 | 1 | \$ 4,525,070.00 | \$ 4,525,070.00 |
| | US 27 TWIN BRIDGES WIDENING (3) | LP SUM | | | | 1 | \$ 1,680,000.00 | \$ 1,680,000.00 |
| | GRAND AVE TWIN BRIDGES WIDENING (3) | LP SUM | | | | 1 | \$ 1,440,000.00 | \$ 1,440,000.00 |
| | CHESAPEAKE AVE TWIN BRIDGES WIDENING (3) | LP SUM | | | | 1 | \$ 1,104,000.00 | \$ 1,104,000.00 |
| | LIGHTING COSTS (4) | LP SUM | | | | 1 | \$ 2,000,000.00 | \$ 2,000,000.00 |
| | SIGNING COSTS (5) | LP SUM | | | | 1 | \$ 200,000.00 | \$ 200,000.00 |
| | | | | SUBTOTAL | \$ 34,719,936.06 | | | \$ 48,787,479.35 |
| 2568 | MOBILIZATION | LP SUM | | | \$ 1,039,581.36 | | | \$ 1,461,147.34 |
| 2569 | DEMobilIZATION | LP SUM | | | \$ 519,790.68 | | | \$ 730,573.67 |
| | TOTAL ESTIMATED CONSTRUCTION COSTS | | | | \$ 36,279,308.10 | | | \$ 50,979,200.36 |
| | 10% ENGINEERING & CONTINGENCIES | | | | \$ 3,627,930.81 | | | \$ 5,097,920.04 |
| | TOTAL ESTIMATED PROJECT COSTS | | | | \$ 39,907,238.91 | | | \$ 56,077,120.40 |

NOTES: (1) Estimated cost of repairs to all ramp pavement within limits of project not including I-275 interchange ramps which are being repaired in a separate project
(2) Includes all bridge repair work recommended by the District within the project limits (see attached spreadsheet for detailed breakdown of costs by bridge number)
(3) Costs for closing up the median on these twin bridges for two additional lanes (see attached spreadsheet for detailed breakdown of costs by bridge number)
(4) Costs to remove existing conventional lighting from raised median and place on new median barrier
(5) Costs to remove existing overhead sign bases from raised median and reconstruct in new median barrier (assumes existing trusses and signs will be used)

I-471 BRIDGE REHABILITATION ESTIMATE
FROM I-275 TO OHIO RIVER
CAMPBELL COUNTY
9/23/2010

| Bridge Maint. No. | Location | Proposed Work | Area (SY) | Area (SF) | LIN FT | Unit Cost | Cost |
|----------------------|---|--|-----------|-----------|--------|-----------|--------------------|
| 019B00049L&R | I-471 SB & NB over US 27 | Seal Decks | 4,510 | | | \$81 | \$365,310 |
| | | Replace Joints | | | 280 | \$700 | \$196,000 |
| | | | | | | Total | \$561,310 |
| 019B00052L&R | I-471 SB & NB over Grand Ave (KY 1892) | Replace Joint Seals | | | 330 | \$50 | \$16,500 |
| | | Patch Spalled Areas Around Joints | | 100 | | \$200 | \$20,000 |
| | | Seal Decks | 3,930 | | | \$81 | \$318,330 |
| | | | | | | Total | \$354,830 |
| 019B00053L&R | I-471 SB & NB over Chesapeake Ave | Replace Joints | | | 268 | \$700 | \$187,600 |
| | | Seal Decks | 3,230 | | | \$81 | \$261,630 |
| | | | | | | Total | \$449,230 |
| 019B00056L&R | I-471 SB & NB over 6 th St | Replace Elastomeric Bearing Pads Are Walking Out On Several Piers; Replace As Needed | 29,900 | | | \$10 | \$299,000 |
| | | SB Piers: 2, 3, 9, 10, 16, 21, 26 NB Piers: 3, 4, 10, 11, 17, 22, 27, 28 | | | | | |
| 019B00065N | I-471 NB Ramp to KY 8 | Replace Joint Seals And Armored Edge Where Necessary | | | 104 | \$700 | \$72,800 |
| 019B00082L&R | I-471 SB & NB over KY 8 Approach Spans to Dan Beard / Big Mac Bridge over Ohio River | Overlay Deck | 5,821 | | | \$100 | \$582,100 |
| | | Replace Joints | | | 390 | \$700 | \$273,000 |
| | | | | | | Total | \$855,100 |
| 019B00039L&R | I-471 SB & NB over Ohio River (Dan Beard / Big Mac Bridges) | Overlay Deck | 16,192 | | | \$100 | \$1,619,200 |
| | | Replace Joints | | | 448 | \$700 | \$313,600 |
| | | | | | | Total | \$1,932,800 |
| Project Total | | | | | | | \$4,525,070 |

**I-471 BRIDGE WIDENING TO CLOSE MEDIAN ESTIMATE
FROM I-275 TO OHIO RIVER
CAMPBELL COUNTY
9/23/2010**

| Bridge Maint. No. | Location | Description | Area (SF) | Unit Cost | Cost |
|--------------------------|--------------------------------------|--------------------|------------------|------------------|--------------------|
| 019B00049L&R | I-471 SB&NB over US 27 | 3 Span Steel | 8,400 | \$200 | \$1,680,000 |
| 019B00052L&R | I-471 SB&NB over Grand Ave (KY 1892) | 2 Span Steel | 7,200 | \$200 | \$1,440,000 |
| 019B00053L&R | I-471 SB&NB over Chesapeake Ave | 3 Span Concrete | 5,520 | \$200 | \$1,104,000 |
| | | | | TOTAL | \$4,224,000 |

Figure E

COMPARISON OF BRIDGE OVERLAY OPTIONS
 1-471 PAVEMENT REHABILITATION
 CAMPBELL COUNTY
 ITEM NO. 6-2021.00

| Bridge | Maint. No. | Location | Area (SF) | Option | | |
|------------------------------|------------|------------------------------|-----------|---------------------|-----------------------------|------------------------|
| | | | | 6" Concrete Overlay | 1.5" Latex Concrete Overlay | Epoxy Urethane Overlay |
| | | | | \$15 | \$8 | \$9 |
| | | | | Option Cost / SF | | |
| | | | | Cost | | |
| B1 | 019B00049L | I-471 SB Over US 27 | 19836 | \$297,540 | \$158,688 | \$178,524 |
| B2 | 019B00049R | I-471 NB Over US 27 | 19836 | \$297,540 | \$158,688 | \$178,524 |
| B3 | 019B00052L | I-471 SB Over Grand Ave | 17217 | \$258,255 | \$137,736 | \$154,953 |
| B4 | 019B00052R | I-471 NB Over Grand Ave | 17217 | \$258,255 | \$137,736 | \$154,953 |
| B5 | 019B00053L | I-471 SB Over Chesapeake Ave | 13419 | \$201,285 | \$107,352 | \$120,771 |
| B6 | 019B00053R | I-471 NB Over Chesapeake Ave | 13419 | \$201,285 | \$107,352 | \$120,771 |
| B7 | 019B00056L | I-471 SB Over 6th St | 0 | \$0 | \$0 | \$0 |
| B8 | 019B00056R | I-471 NB Over 6th St | 0 | \$0 | \$0 | \$0 |
| B9 | 019B00065N | I-471 NB Ramp To KY 8 | 0 | \$0 | \$0 | \$0 |
| B10 | 019B00082L | I-471 SB Over KY 8 | 29196 | \$437,940 | \$233,568 | \$262,764 |
| B11 | 019B00082R | I-471 NB Over KY 8 | 23463 | \$351,945 | \$187,704 | \$211,167 |
| B12 | 019B00039L | I-471 SB Over Ohio River | 66627 | \$999,405 | \$533,016 | \$599,643 |
| B13 | 019B00039R | I-471 NB Over Ohio River | 66627 | \$999,405 | \$533,016 | \$599,643 |
| TOTAL COST PER OPTION | | | | \$4,302,855 | \$2,294,856 | \$2,581,713 |